

LINE & CURVE TABLES

Line Table

Line #	Direction	Length			
L1	N41° 47' 41"E	23.25'			
L2	N20° 49' 34"W	36.32'			
L3	N84° 20' 43"W	47.00'			
L4	N47° 34' 38"E	59.14'			
L5	N16° 28' 30"E	7.72'			
	:				

Curve Table

Curve #	Radius	Length	Delta
C1	65.00'	71.10'	62° 40' 27"
C2	28.00'	31.04'	63° 31' 09"
C3	54.00'	50.17'	53° 13' 54"

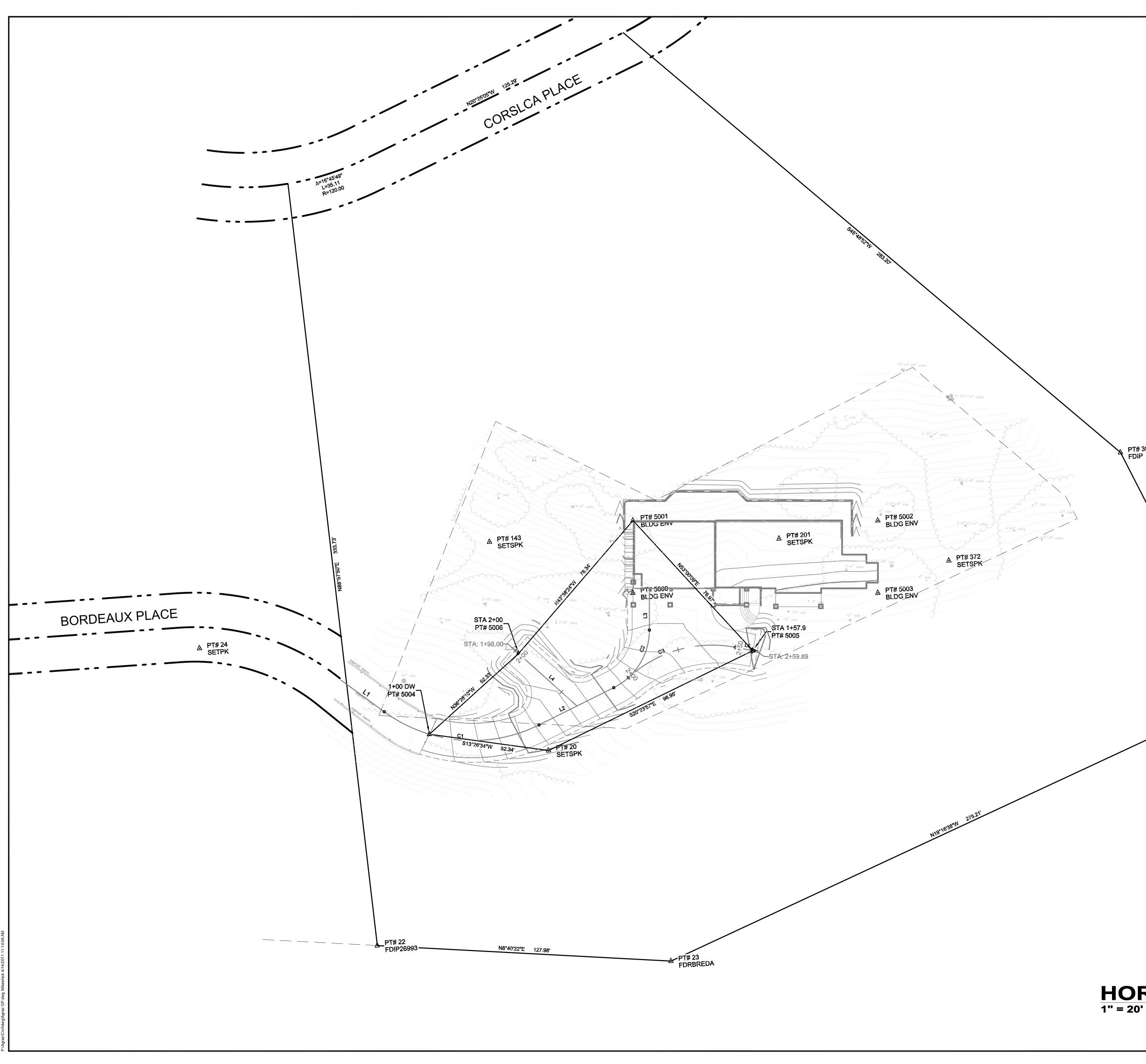




20'

SHEET NUMBER:

C-2.1



LINE & CURVE TABLES

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L1	N41° 47' 41"E	23.25'			
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BASIS OF COORDINATES & ELEVATION

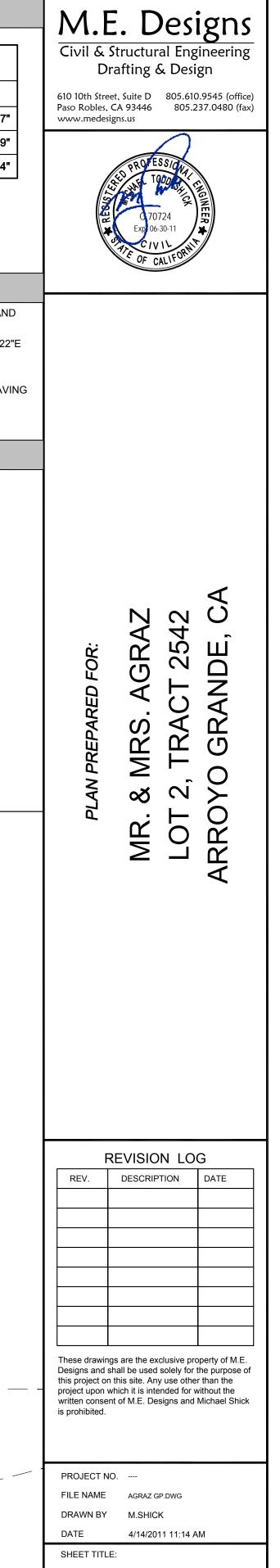
THE BOUNDARY LINES SHOWN ARE BASED ON A PARTIAL BOUNDARY SURVEY AND RECORD CALCULATIONS. THE BASIS OF BEARINGS IS BETWEEN THE FOUND MONUMENTS ON THE EAST LINE OF LOT 2 OF TRACT 2542 AND NOTED AS N8°40'22"E AS SHOWN ON 29 MB 35.

THE ELEVATIONS SHOWN ARE BASED ON THE TOP OF A SET SPIKE (#20) AND HAVING AN ASSUMED ELEVATION OF 203.54'.

HORIZONTAL CONTROL

Point #	Northing	Easting	Elevation	Description
20	4955.9927	4903.5974	100.0000	SETSPK
22	4873.4834	4980.7017	83.5506	FDIP26993
23	5000.0118	5000.0018	91.5365	FDRBREDA
24	4809.2568	4844.3808	96.4052	SETPK
143	4939.3900	4810.7545	112.0162	SETSPK
201	5064.9214	4821.7448	120.5113	SETSPK
372	5137.5229	4838.5376	116.5370	SETSPK
392	5216.4692	4799.0991	125.5976	FDIP
5000	4999.3176	4838.9832	0.0000	BLDG ENV
5001	5002.4214	4807.6365	0.0000	BLDG ENV
5002	5108.4860	4818.1383	0.0000	BLDG ENV
5003	5105.3823	4849.4850	0.0000	BLDG ENV
5004	4905.0873	4891.4299	0.0000	1+00 DW
5005	5048.7386	4869.1072	0.0000	STA 1+57.9
5006	4947.1716	4860.3239	0.0000	STA 2+00





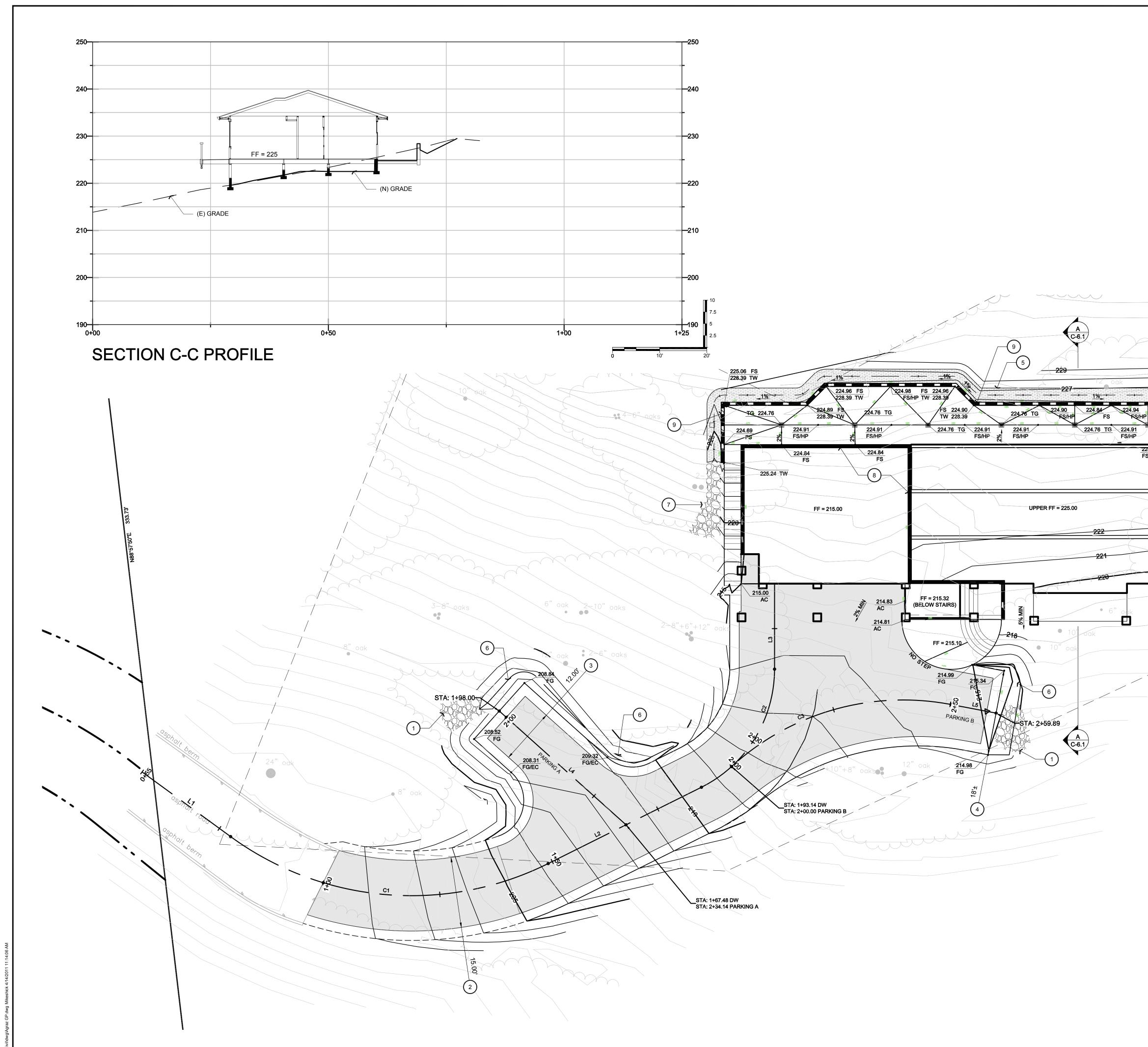


C-2.2

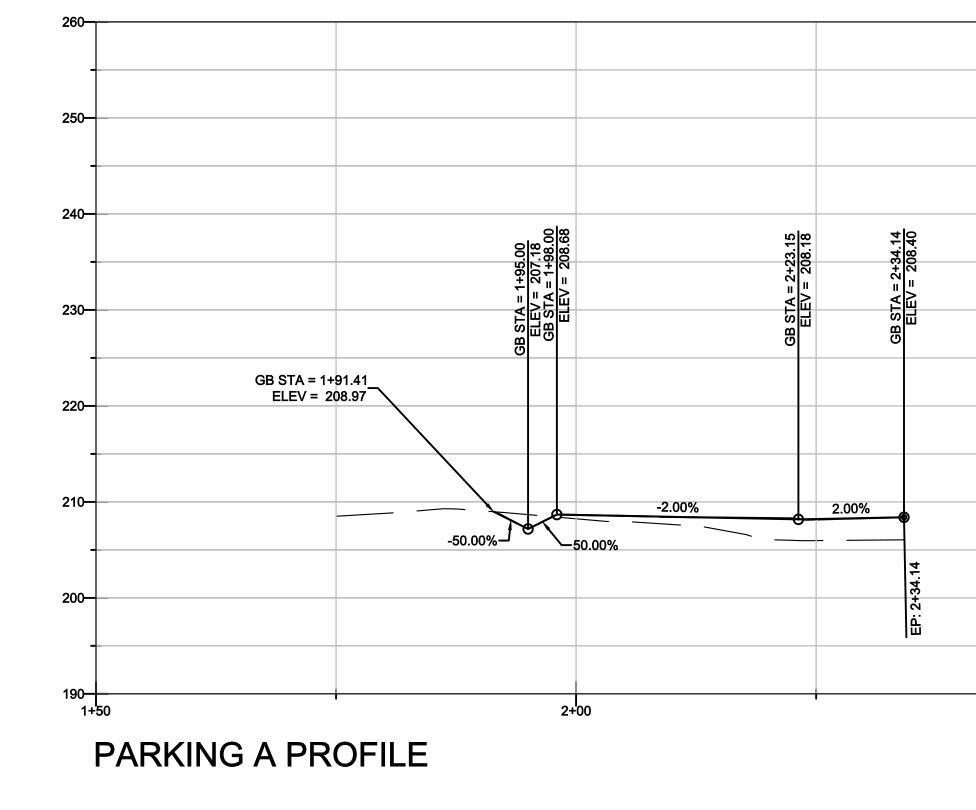
SHEET NUMBER:

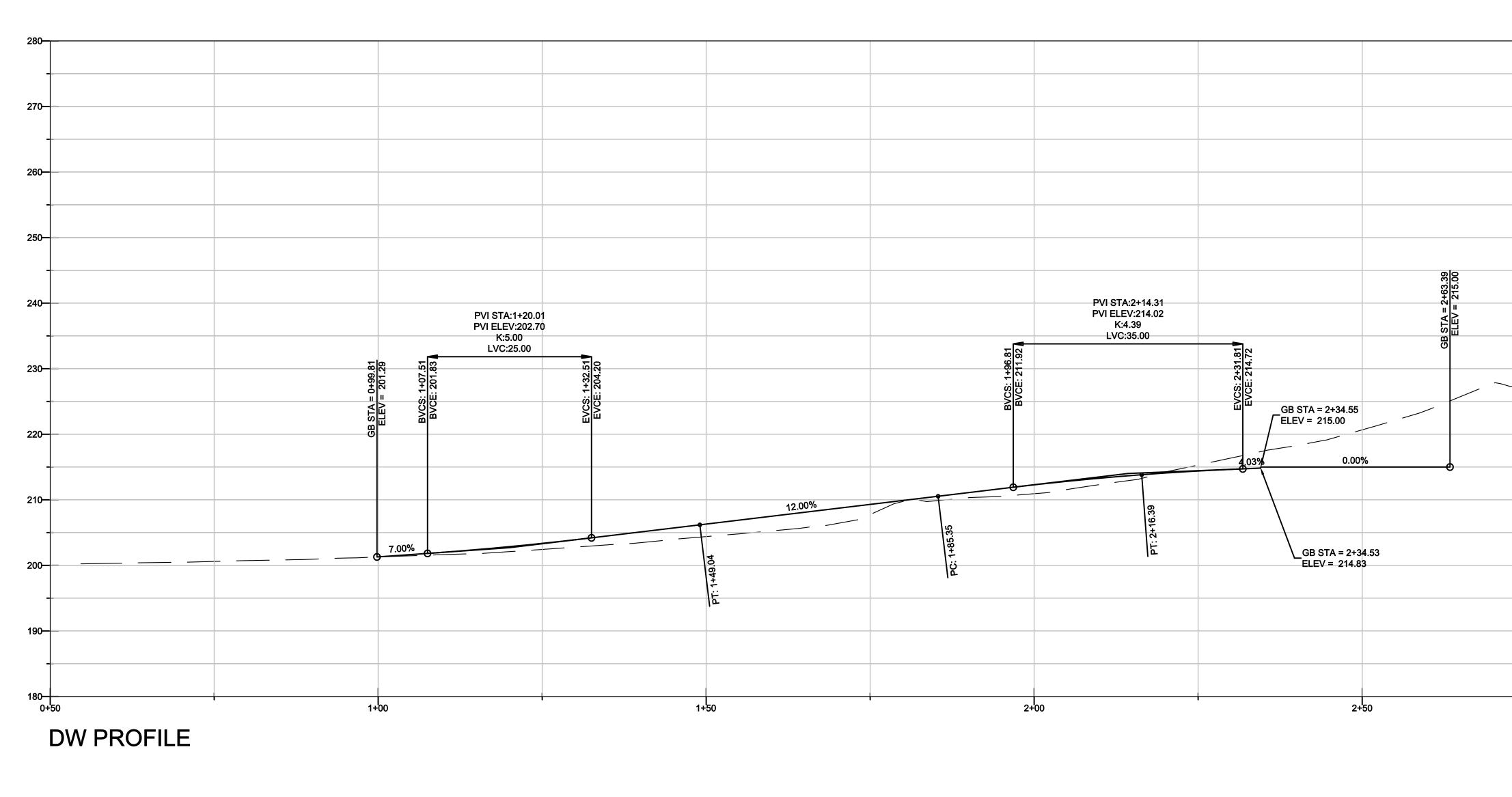


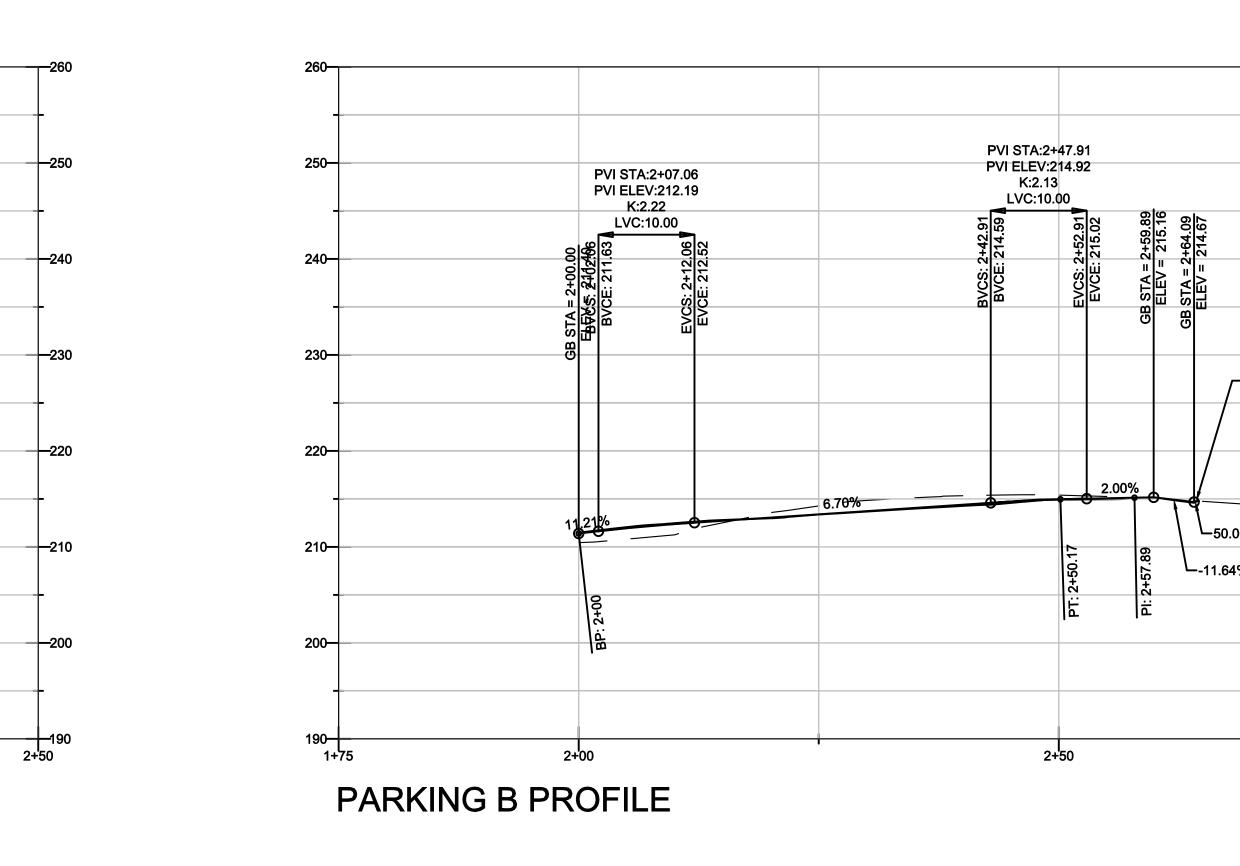
HORIZONTAL CONTROL 0 20' 40'



	\square	REFERENCE NOTES		Decienc	
	3. 4. 5. 6. 7. 8. 9.	· · · · · · · · · · · · · · · · · · ·	M.E. Designs Civil & Structural Engineering Drafting & Design 610 10th Street, Suite D Paso Robles, CA 93446 www.medesigns.us 805.610.9545 (office) 805.237.0480 (fax) WWW.medesigns.us		
224.84 5% MIN 5% MIN 3-6" och		2 - 6" + 4" oak 12" oak FS 2 - 6" + 2 - 4" oaks 00 FG 70 TW 7 oak 7 oak	PLAN PREPARED FOR:	MR. & MRS. AGRAZ LOT 2, TRACT 2542 ARROYO GRANDE, CA	
		CRADING PLANT= 10'	REV.	AGRAZ GP.DWG M.SHICK 4/14/2011 11:14 AM T OING PLAN COSS TION	
				C-3.1	







	-280	M.E. Designs Civil & Structural Engineering Drafting & Design
		610 10th Street, Suite D Paso Robles, CA 93446 www.medesigns.us 805.237.0480 (fax)
	-260	PROFESSION TRADE
	-250	
	-240	
	-230	
	-220	
	-200	^{FOR:} GRAZ 12542 IDE, (
		PLAN PREPARED FOR: . & MRS. AGR T 2, TRACT 25 OYO GRANDE
	-190	ргамря & М Т 2, Т ОҮО
3.	+180 +00	PLAN PREPARED FOR: MR. & MRS. AGRAZ LOT 2, TRACT 2542 ARROYO GRANDE, CA
	260	
		REVISION LOG
	240	REV. DESCRIPTION DATE
GB STA = 2+64.33 ELEV = 214.79	230	These drawings are the exclusive property of M.E. Designs and shall be used solely for the purpose of
	220	besigns and shan be used solery for the purpose of this project on this site. Any use other than the project upon which it is intended for without the written consent of M.E. Designs and Michael Shick is prohibited.
00% 4%	210	PROJECT NO FILE NAME AGRAZ GP.DWG
	200	DRAWN BY M.SHICK DATE 4/14/2011 11:14 AM SHEET TITLE:
		DRIVEWAY PROFILES
DRIVE	WAY PROFLES	SHEET NUMBER:
1 " = 10'		C-3.2

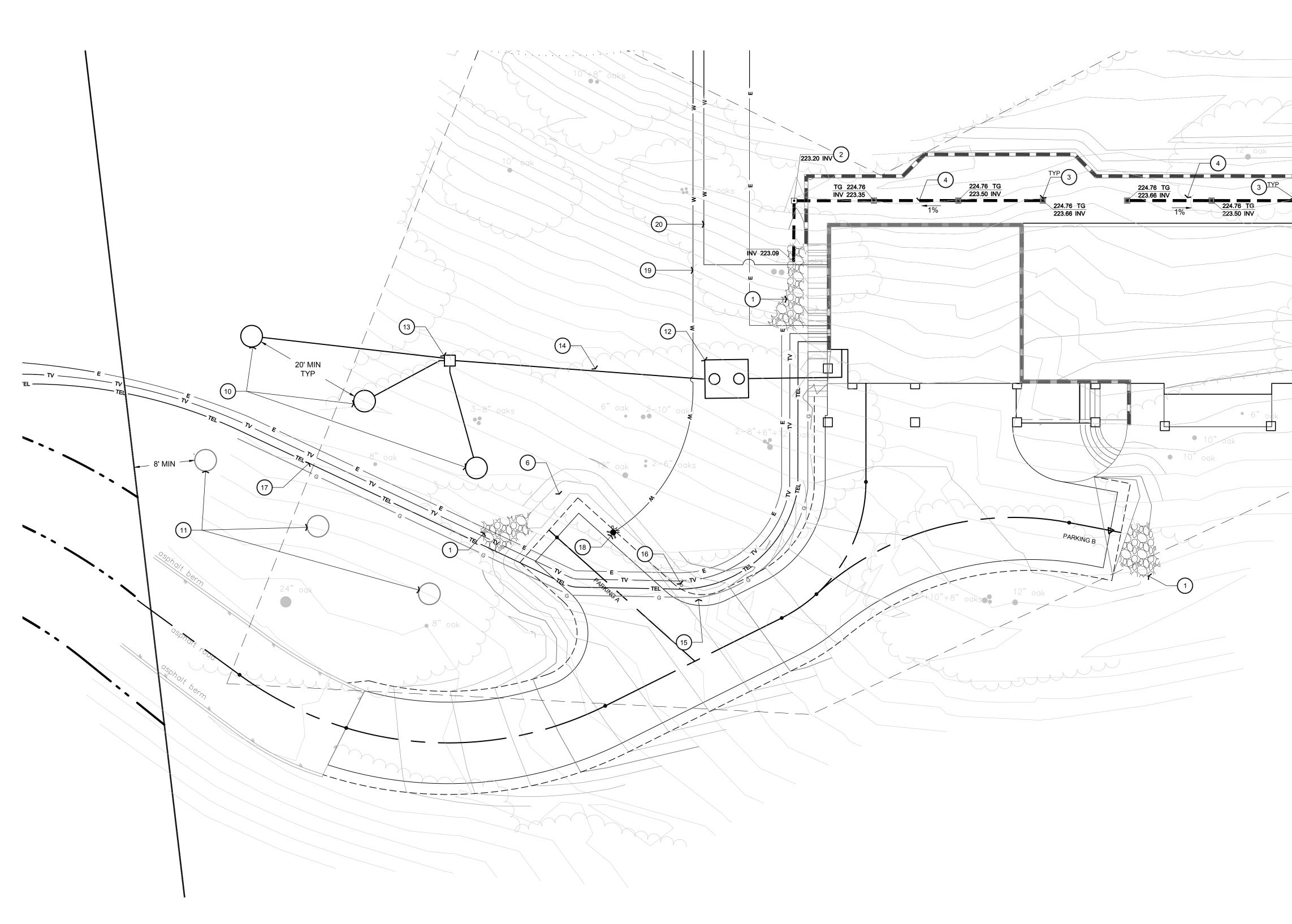
SEWAGE DISPOSAL SYSTEM SETBACKS

MIN DISTANCE REQUIRED FROM	BUILDING SEWER	SEPTIC TANK	DISPOSAL FIELD
BUILDINGS/STRUCTURES	2 FEET	5 FEET	8 FEET
PROPERTY LINE	CLEAR	5 FEET	5 FEET
WATER SUPPLY WELLS	50 FEET	50 FEET	100 FEET
STREAMS/ SPRINGS & WATER MIGRATION	50 FEET	50 FEET	100 FEET
LARGE TREES	-	10 FEET	-
RESERVIOR, SPILLWAY ELEVATION	-	200 FEET	200 FEET
DISPOSAL FIELD	-	5 FEET	4 FEET ^{1, 2}
DOMESTIC WATER LINE	1 FOOT	5 FEET	5 FEET
DOMESTIC WELLS	-	50 FEET	100 FEET
DISTRIBUTION BOX	-	-	5 FEET

(1) PLUS TWO FEET FOR EACH ADDITIONAL FOOT OF TRENCH DEPTH IN EXCESS OF ONE FOOT BELOW BOTTOM OF LEACH PIPE (2) SEPTIC PITS SHALL BE SEPERATED BY 20 FEET (WALL TO WALL)

SEPTIC NOTES

- 1. SEPTIC TANK CAPACITY SHALL BE AT LEAST 1500 GALLONS. A NEW STANDARD TWO COMPARTMENT CONCRETE OR FIBERGLASS SEPTIC TANK SHALL BE INSTALLED.
- 2. ALL PIPING TO AND FROM THE SEPTIC TANK SHALL BE 4 INCH ASTM APPROVED SEWER PIPE (SRD35PVC). MINIMUM PIPE SLOPE SHALL BE 2%. SANITARY TEES SHALL BE PLACED AND SEALED (GROUT, SILICONE OR FOAM) INSIDE TANK INLET AND OUTLET.
- 3. VERTICAL PIPING IN SEEPAGE PITS SHALL BE 4 IN PERFORATED ASTM APPROVED PIPE. THE PIPE IS TO BE CAPPED ON THE BOTTOM AND PLACED AS CLOSE AS POSSIBLE IN THE CENTER OF THE PIT BORING FROM THE PIT BOTTOM TO THE TOP OF THE LEACH GRAVEL.
- 4. GRAVEL DEPTH MUST NOT EXCEED THE DEPTH NECESSARY TO MAINTAIN 15 FEET TO "DAYLIGHT" MEASURED HORIZONTALLY FROM THE TOP OF THE INFILTRATIVE SURFACE (APPROX. 4 FT).
- 5. FILTER FABRIC SHALL BE PLACED OVER GRAVEL PRIOR TO BACKFILLING.
- 6. FOUR INCH INSPECTION RISERS WITH REMOVABLE CAPS SHALL BE INSTALLED AT GROUND SURFACE (OR ACCESSIBLE AT THE SURFACE) ON EACH PIT AS SHOWN.



- 7. PITS SHALL BE PLACED AT LEAST 20 FEET APART (SIDE TO SIDE).
- 8. SYSTEM INSTALLATION SHALL BE INSPECTED BY CITY OFFICIALS AND DESIGN ENGINEER PRIOR TO BACKFILLING. A MINIMUM OF 48 HOUR PRIOR NOTICE IS REQUIRED.
- 9. FINISH GRADING SHALL DIRECT ALL SURFACE RUNOFF AROUND THE LEACH AREA, AS INDICATED BY ARROWS.
- 10. ALL UTILITY COMPANIES TO BE NOTIFIED PRIOR TO START OF CONSTRUCTION BY THE CONTRACTOR BY CALLING 1-800-642-2444.
- 11. SINCE PERCOLATION TEST DEPTH WAS ONLY 5' DEEP, THE FIRST PIT INSTALLED MUST BE OVER-EXCAVATED TO AT LEAST 50' DEEP TO ENSURE NO GROUNDWATER OR IMPERMEABLE LAYER EXISTS. THE HOLE WILL THEN BE FILLED WITH NATIVE MATERIAL PER GEOTECHNICAL ENGINEER SUCH THAT ONLY 35' OF ROCK DEPTH EXISTS (40' MAX PIT DEPTH).

SEPTIC DESIGN

DESIGN BASIS:

3 OR 4 BEDROOMS 44 MIN/IN, PERCOLATION RATE

APPLICATION RATE (q) = 0.3 (GPD/SQ FT) FLOW (Q) = 375 (GALLONS/DAY)

LEACH AREA (A) : A = Q/q = 400 GPD/0.3 GPD/SQ FT = 1333.3 SQ FT

PIT DESIGN:

3

4

A = πDH D = PIT DIA (FT) H = ROCK DEPTH IN PIT (FT) H = A/(πD)

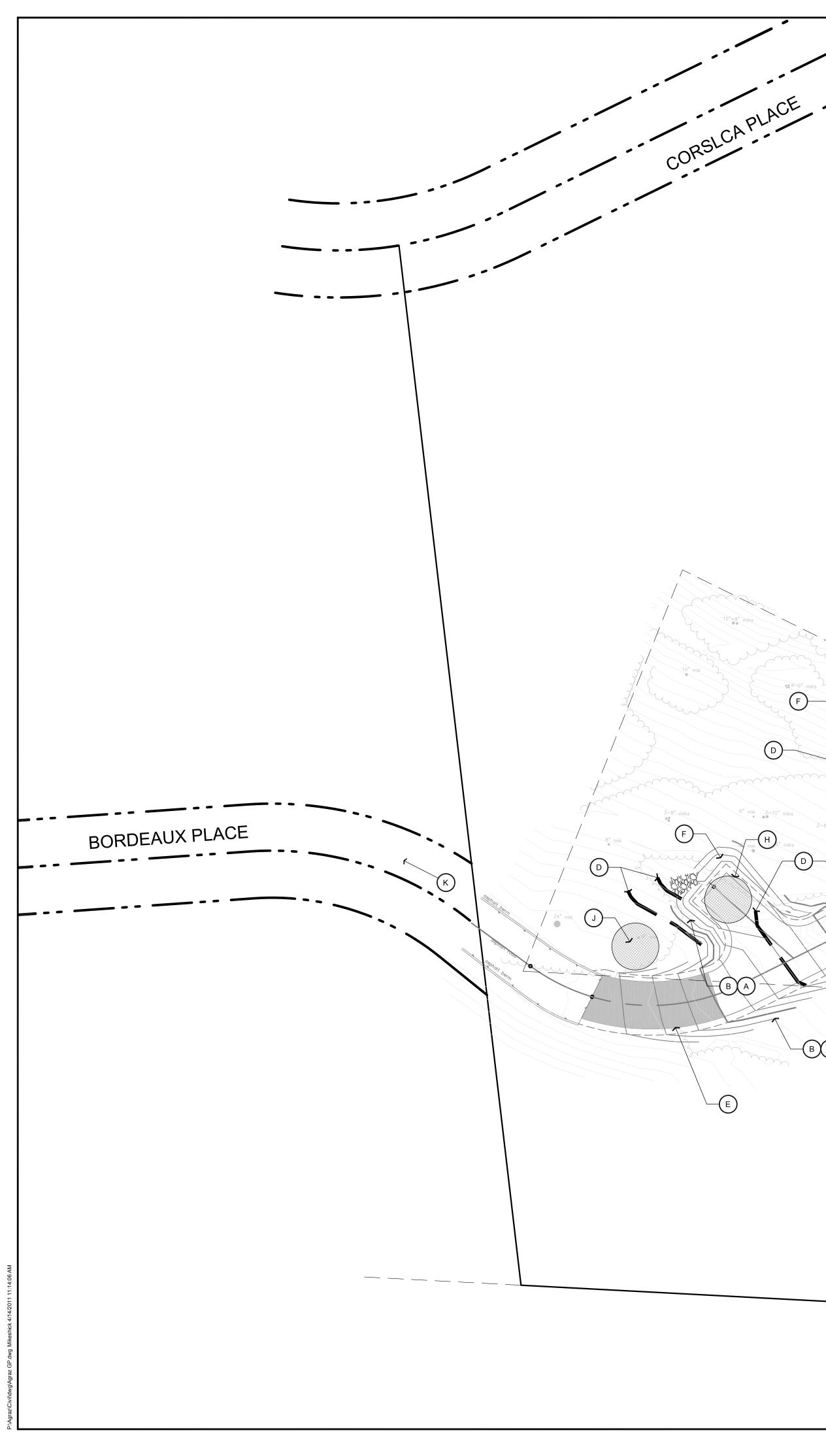
H (D=4') = 106' EFFECTIVE PIT DEPTH TOT PIT DIA (FT) NO OF PITS (ROCK DEPTH) (FT) 53 2 4 35

27

4

4

	\bigcirc	REFERENCE NOTES	
	6. 7. 8. 9.	SLOPE PROTECTION PER SHEET C-3.1 12x12 NDS CATCH BASIN WITH SUMP BOX LID (OR EQUAL) 12x12 NDS CATCH BASIN WITH FLAT GRATE (OR EQUAL) 6" PVC SCHED 40 STORM DRAIN PIPE. SLOPE = 1% MIN OKAY TO CONNECT GUTTER SPOUTS TO STORM DRAIN AROUND HOUSE. PROVIDE ADEQUATE SLOPE PROTECTION AT ALL OUTLETS GAS	M.E. Designs Civil & Structural Engineering Drafting & Design 610 10th Street, Suite D Paso Robles, CA 93446 www.medesigns.us 805.610.9545 (office) 805.237.0480 (fax)
AL PIT DEPTH (FT) 55 40 30	 11. 12. 13. 14. 15. 16. 17. 18. 19. 	 (b) CLI THE THE, TO BELL THIN OF OF THE ONE ON THE ONE ON THE ONE ON THE THE ONE ON THE ONE ON THE O	G 70724 Exp 06-30-11 C IV IL FF OF CALIFORNIT
224.76 TG		$2-6^{n}+4^{n}$ ock	PLAN PREPARED FOR: MR. & MRS. AGRAZ LOT 2, TRACT 2542 ARROYO GRANDE, CA
			REVISION LOG REV. DESCRIPTION DATE DESCRIPTION DATE
	JT "= ′	<image/> TILITY / SEPTIC PLAN	PROJECT NO FILE NAME AGRAZ GP.DWG DRAWN BY M.SHICK DATE 4/14/2011 11:14 AM SHEET TITLE: UTILITY & SEPTIC PLAN SHEET NUMBER: C-4.1



STORMWATER QUALITY PLAN NOTES THE PROPOSED PROJECT IS LOCATED OFF HIGHWAY 227 (CARPENTER CANYO EDNA HILLS ESTATES. THIS DEVELOPMENT IS NESTLED IN EASTERLY FACING COMPLETE WITH MULTIPLE NATIVE TREES, GRASSES, AND OTHER VEGETATION PROPOSED FOR THIS PROJECT, LOT 2, IS CONSISTENT WITH THE CHARACTER DEVELOPMENT. IT IS HEAVILY WOODED WITH MATURE OAK TREES AND CONT. SUPPLY OF NATURE GRASS AND VEGETATION. THE PROPOSED DESIGN IS INTENDED TO PROVIDE MINIMAL SITE DISTURBANC INCORPORATE AS MUCH NATURAL LANDSCAPE AS POSSIBLE. SITE GRADING MINIMUM USING A CUT-IN STYLE GARAGE AND RAISED FLOOR SYSTEM FOR TH OF THE RESIDENCE. THE ACCESS DRIVEWAY WAS DESIGNED TO INTEGRATE NATURAL CONTOURS AS MUCH AS POSSIBLE, LEAVING VERY LITTLE DISTURBA HISTORICAL DRAINAGE RUNOFF. THE RUN-OFF OF GREATEST SIGNIFICANCE TO THIS PROJECT IS THE WATER U THE RESIDENCE BEING DIVERTED AROUND EACH SIDE OF THE RESIDENCE AN DRAINAGE ALSO BEING DIVERTED TO EACH SIDE OF THE RESIDENCE. THE FEATURES INTENDED TO CONTROL THE STATED RUN-OFF INCLUDE A 3-FC DITCH BEHIND A SITE RETAINING WALL, LINED WITH EROSION CONTROL MAT V FLOW INTO RIP RAP LINED SWALES, WHERE THE DRAINAGE IS INTENDED TO P NATURALLY ONSITE. ALL DISTURBED AREAS SHALL BE HYDROSEEDED AND LA CLEAN AND SLOW DEVELOPED FLOW TO A RATE THAT WILL PERMIT MINIMAL S THE DESIGN AND PROPOSED LOCATION SHALL TAKE ADVANTAGE TO THE QUA NATURAL LANDSCAPE AND NEARLY ELIMINATE DEVELOPED FLOW FROM LEAV -(B)A2-6"+4" oc F -(F)

ON RD IN THE 6 HILL SIDE AND ON. THE LOT R OF THE ENTIRE TAINS A LARGE	 A. HYDROSEED ALL CUT AND FILL SLOPES PER EROSION CONTROL NOTES (THIS SHEET) B. LINE ALL BARE SLOPES WITH C125 NORTH AMERICAN GREEN EROSION CONTROL BLANKET PER DETAIL 11/C-6.2 C. SILT FENCING PER DETAIL 22/C-6.2 (AS NEEDED) D. STRAW WATTLE PER DETAIL 31/C-6.2 	M.E. Designs Civil & Structural Engineering Drafting & Design 610 10th Street, Suite D Paso Robles, CA 93446 www.medesigns.us
CE AND SHALL WAS KEPT TO A HE REMAINDER WITH THE BANCE TO	 E. TEMPORARY GRAVEL ENTRANCE PER DETAIL 21/C-6.2 F. LINE SWALES PER 21/C-6.1 & 12/C-6.2 G. BROW DITCH PER GRADING PLAN, C-3.1 H. STOCKPILE MANAGE PER 32/C-5.1+ I. HAZARDOUS MATERIAL CONTAINMENT PER 11/C-6.3 J. CONCRETE WASHOUT AREA PER 22/C-6.3 K. PROVIDE STORM DRAIN INLET PROTECTION PER 12/C-6.3 AS NEEDED DOWNSTREAM 	0.70724 Exp. 06-30-11
UPHILL FROM ND THE ROOF	EROSION CONTROL NOTES	OF CALIFORNIE
OOT WIDE BROW WHICH CHANNELS PERCOLATE ANDSCAPED SITE RUN-OFF. ANTITY OF VING THE SITE.	 ALL CUT OR FILL SLOPES TO BE PLANTED OR HYDRO-MULCHED IMMEDIATELY AFTER COMPACTION TO PREVENT EROSION SEDIMENTATION AND EROSION CONTROL MEASURES, AS APPROVED BY THE BUILDING OFFICIAL, SHALL BE IN PLACE BETWEEN THE DATES OF OCTOBER 15TH THROUGH APRIL 15TH WHERE GRADING MAY AFFECT OFF-SITE PROPERTY. SHOULD THESE MEASURES NOT BE IN PLACE NO INSPECTIONS WILL BE GIVEN UNTIL MEASURES ARE IMPLEMENTED [UBC 3316] THE CONTRACTOR SHALL BE RESPONSIBLE FOR IMPLEMENTING & MONITORING THE APPROVED EROSION & SEDIMENTATION CONTROL PLAN ALL CUT & FILL SLOPES SHALL BE HYDROSEEDED WITH THE FOLLOWING SEED MIX. CUT SLOPES HIGHER THAN 8 FEET WILL REQUIRE JUTE TO COVER THE SEEDS. NATIVE PLANTS SHOULD ALSO BE PLANTED IN THESE AREAS 	
	SEED MIX LUPINUS SUCCULENTUS, LUPINUS NANUS, ESCHSCHOLZIA CALIFORNICA, GILA CAPITATA, & CLARKIA UNGUICULATA. A HYDROMULCH SHALL BE USED	
	TO COVER THE SEED.	PLAN PREPARED FOR: MR. & MRS. AGRAZ LOT 2, TRACT 2542 ARROYO GRANDE, CA
		<form></form>
		PROJECT NO FILE NAME AGRAZ GP.DWG DRAWN BY M.SHICK DATE 4/14/2011 11:14 AM SHEET TITLE: EROSION CONTROL PLAN
EROSI 1'' = 20'		SHEET NUMBER: C-5.1

NOTE FROM SOILS ENGINEER

DUE TO THE TOPOGRAPHY OF THE INDIVIDUAL L BUILDING SITES AND THE PRESENCE OF LOOSE SURFACE SOILS, IT IS ANTICIPATED THAT ENGINEERED FILL PADS WILL BE DEVELOPED FOR THE PROPOSED RESIDENCES WITH FOUNDATION SYSTEMS CONSISTING OF SHALLOW FOOTINGS EXCAVATED INTO ENGINEERED FILL. SINCE WEATHERED BEDROCK WAS ENCOUNTERED AT 4 TO 9 FEET BELOW GROUND SURFACE (BGS) IN A MAJORITY OF THE BORINGS, AS AN ALTERNATIVE TO CONSTRUCTING ENGINEERED FILL PADS, DEEPENED FOUNDATIONS WITH ALL FOOTINGS EXTENDING INTO UNIFORM COMPETENT WEATHERED BEDROCK AS OBSERVED AND APPROVED BY A REPRESENTATIVE OF GEOSOLUTIONS, INC. MAY BE CONSIDERED FOR EACH RESIDENCE. DEEPENED FOOTINGS MAY BE REQUIRED IN CERTAIN AREAS TO ACHIEVE THE REQUIRED EMBEDMENT DEPTH IN UNIFORM COMPETENT WEATHERED BEDROCK. ALL FOUNDATIONS ARE TO BE EXCAVATED INTO UNIFORM COMPETENT MATERIAL TO LIMIT THE POTENTIAL FOR DISTRESS OF THE FOUNDATION SYSTEMS DUE TO DIFFERENTIAL SETTLEMENT. IF CUTS STEEPER THAN ALLOWED BY STATE OF CALIFORNIA CONSTRUCTION SAFETY ORDERS FOR "EXCAVATIONS, TRENCHES, EARTH WORK ARE PROPOSED, A NUMERICAL SLOPE STABILITY ANALYSIS MAY BE NECESSARY FOR TEMPORARY CONSTRUCTION SLOPES.

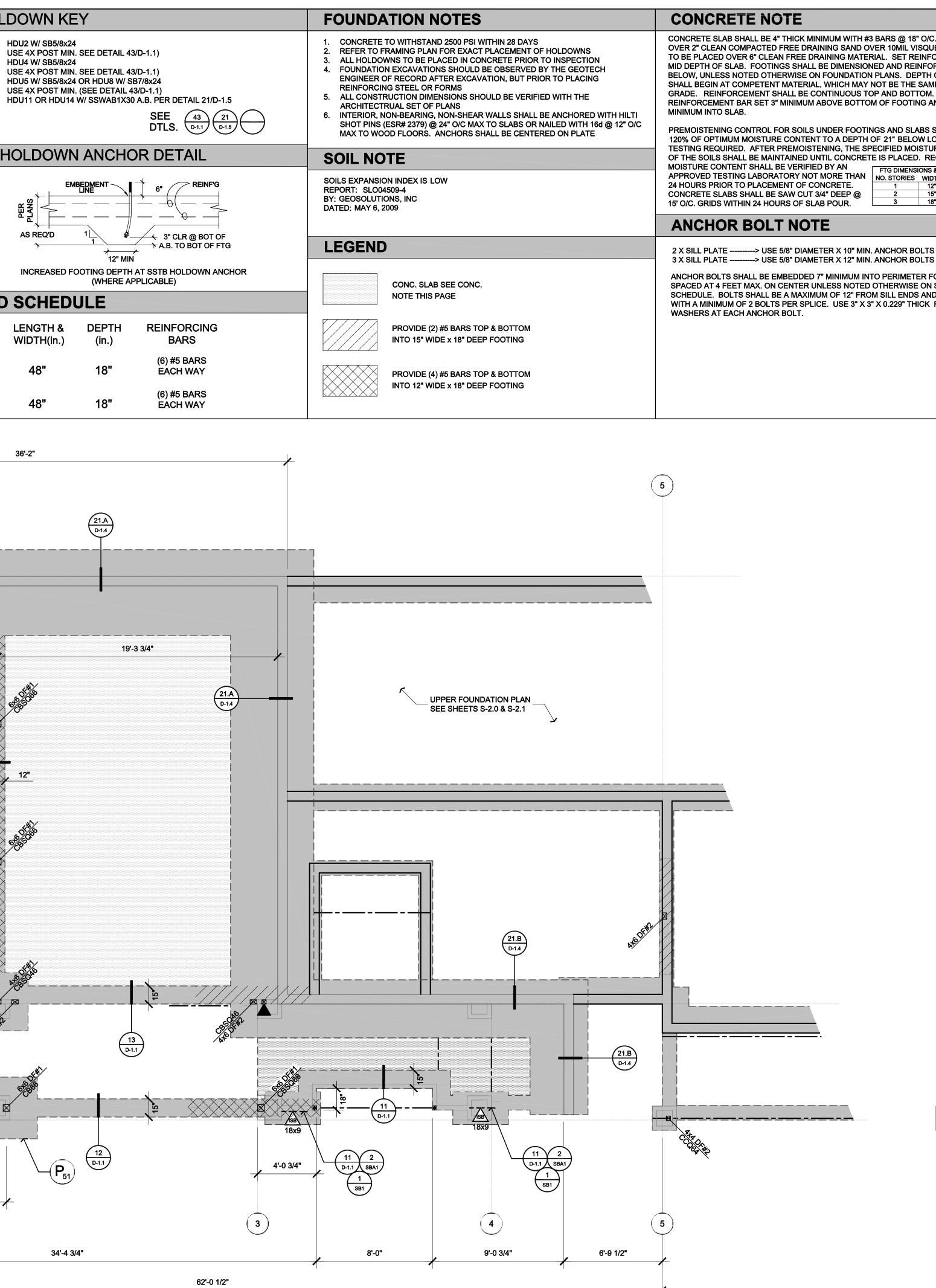
FOR SLAB-ON-GRADE CONSTRUCTION WITH FOOTINGS FOUNDED A MINIMUM OF 12 INCHES INTO UNIFORM COMPETENT FORMATIONAL MATERIAL, THE PAD AREA TO RECEIVE SLAB-ON-GRADE CONSTRUCTION SHOULD BE GRADED SUCH THAT ALL SLABS ARE SUPPORTED UNIFORM COMPETENT MATERIAL. THE FORMATIONAL MATERIAL SHOULD BE OVER-EXCAVATED BENEATH THE SLAB AT LEAST 12 INCHES BELOW EXISTING GRADE AND FINISHED SLAB ELEVATION, TO COMPETENT FORMATIONAL MATERIAL OR ONE-HALF THE DEPTH OF THE DEEPEST FILL; WHICHEVER IS GREATEST. THE EXPOSED SURFACE SHOULD LD BE SCARIFIED TO A DEPTH OF 12 INCHES, MOISTURE CONDITIONED TO NEAR OPTIMUM MOISTURE CONTENT AND COMPACTED TO A MINIMUM RELATIVE DENSITY OF 90 PERCENT (ASTM D1557-07). REFER TO FIGURE 4 & APPENDIX C OF SOILS REPORT FOR FILL PLACEMENT.

PAVEMENT AREAS SHOULD BE OVER-EXCAVATED 12 INCHES BELOW EXISTING GRADE OR FINISHED SUBGRADE; WHICHEVER IS DEEPER. THE EXPOSED SURFACE SHOULD BE SCARIFIED AN ADD ITIONAL DEPTH OF 8 INCHES, MOISTURE COND ITIONED TO NEAR OPTIMUM MOISTURE CONTENT AND COMPACTED TO A MINIMUM RELATIVE DENSITY OF 90 PERCENT (ASTM D1557-07 TEST METHOD). THE OVEREXCAVATED SOIL SHOULD THEN BE MOISTURE CONDITIONED TO PRODUCE A WATER-CONTENT OF AT LEAST 1 TO 2 PERCENT ABOVE OPTIMUM VALUE AND THEN COMPACTED TO A MINIMUM RELATIVE DENSITY OR 90 PERCENT. THE TOP 12 INCHES OF SUB-GRADE SOIL UNDER ALL PAVEMENT SEC IONS SHOULD BE COMPACTED TO A MINIMUM RELATIVE DENSITY OF 95 PERCENT BASED ON THE ASTM D1557-07 TEST METHOD AT SLIGHTLY ABOVE OPTIMUM

HOLDOWN KEY

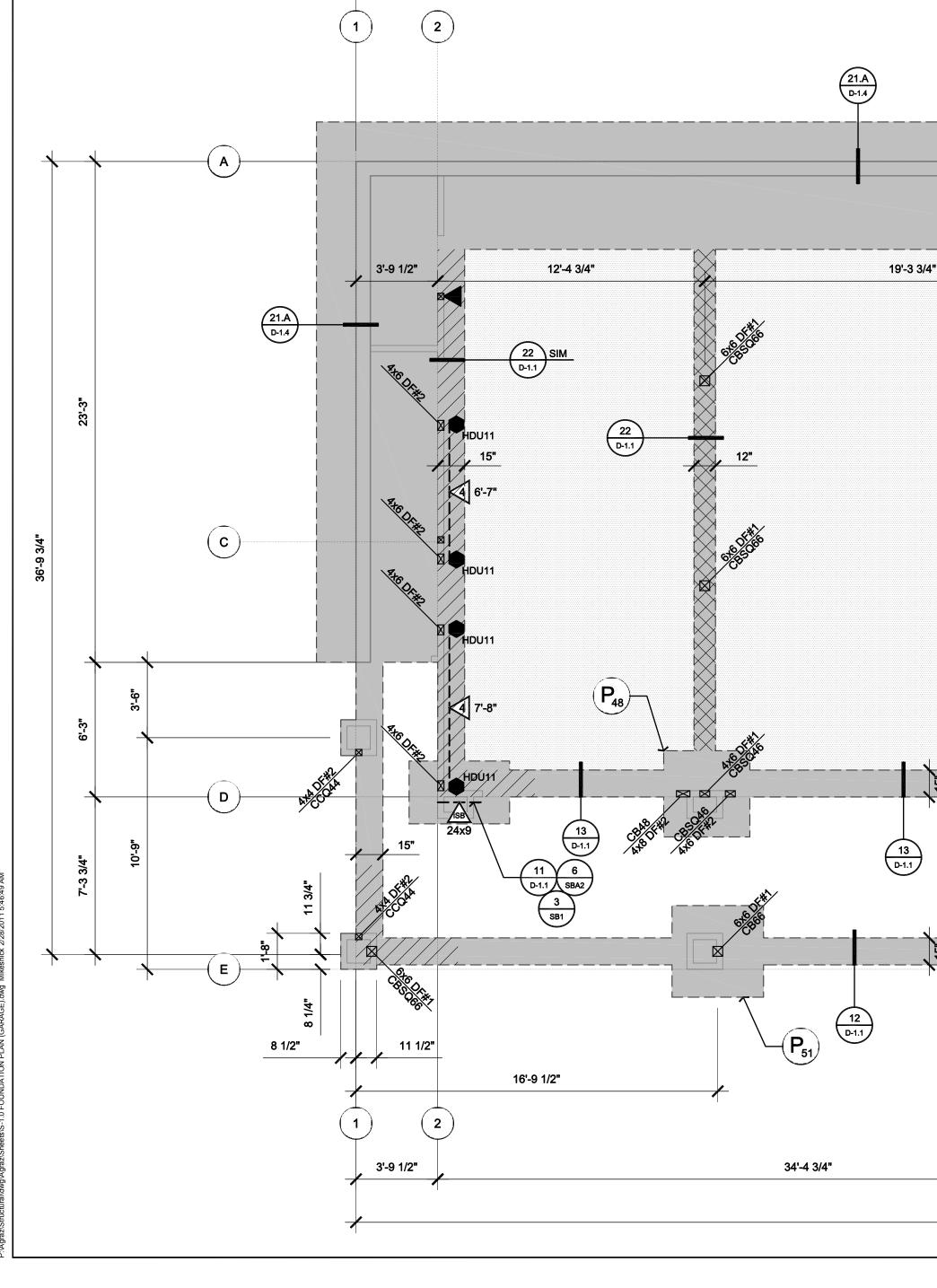
- ▼= HDU2 W/ SB5/8x24

SB HOLDOWN ANCHOR DETAIL

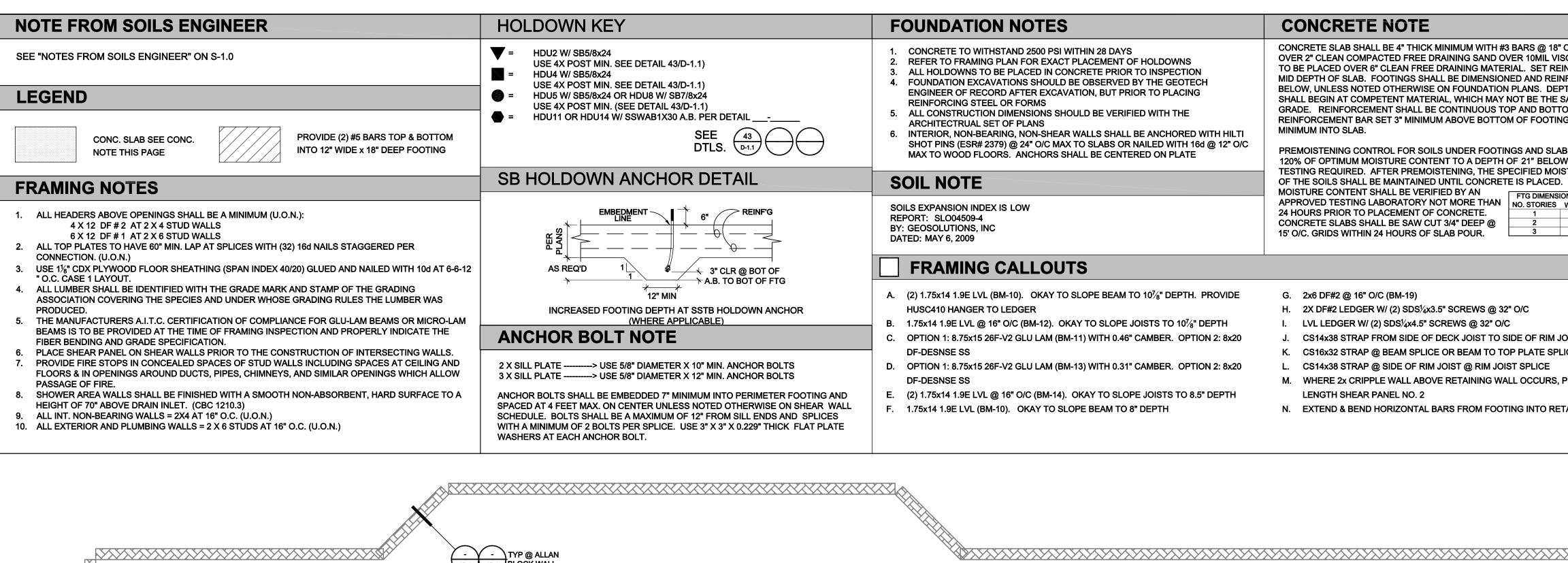


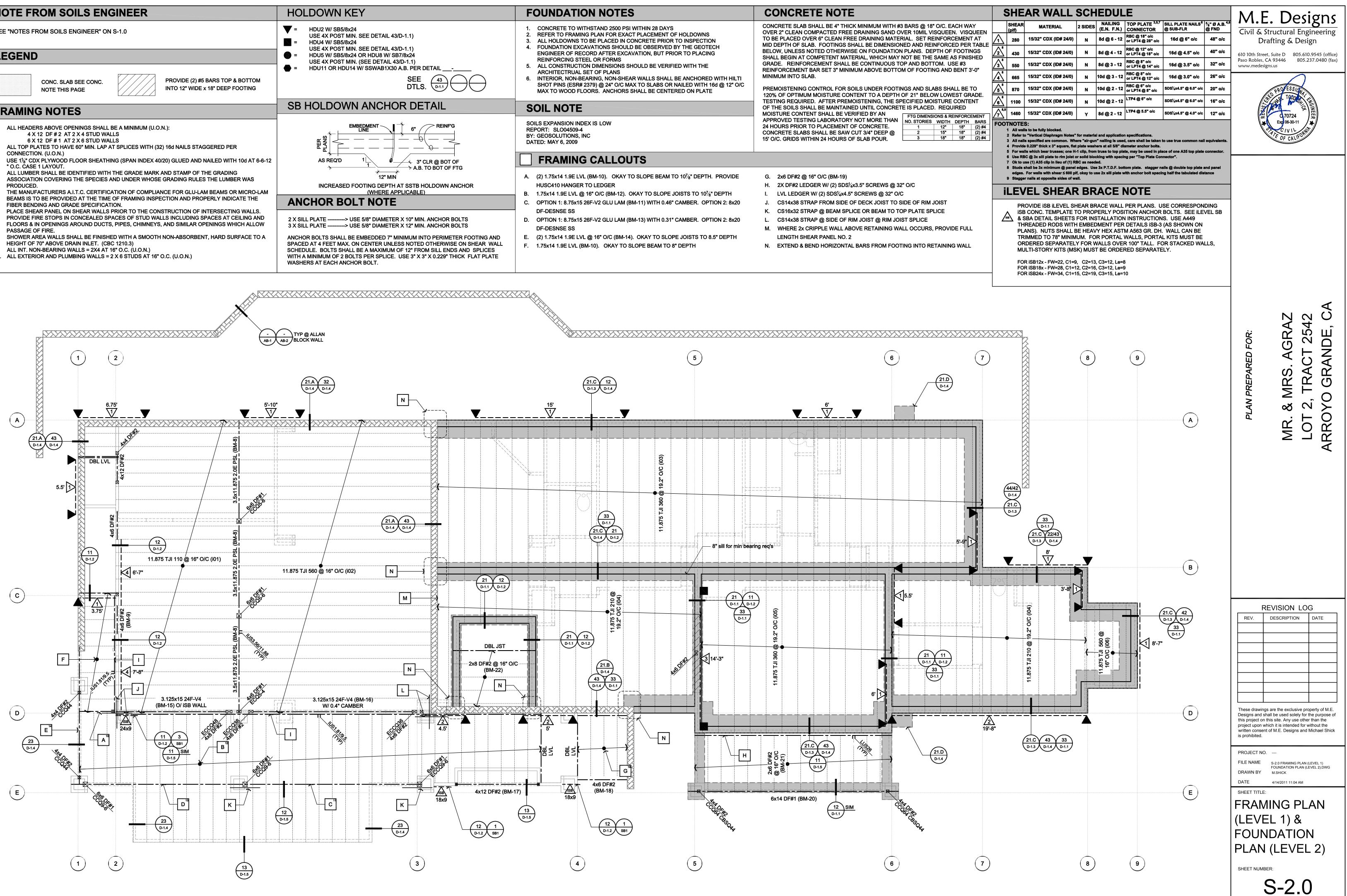
PAD SCHEDULE

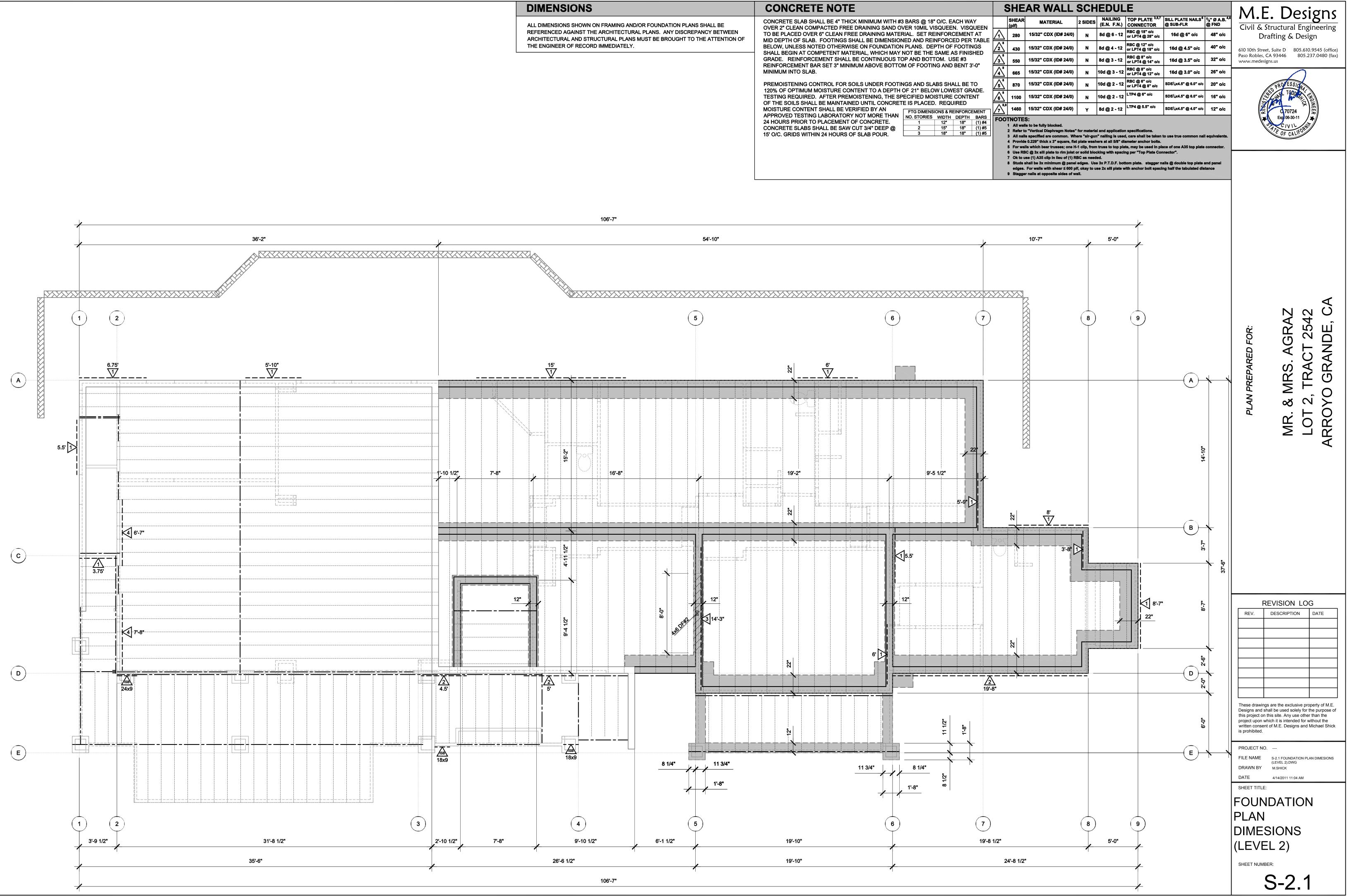
(P _#)	LENGTH & WIDTH(in.)	DEPTH (in.)	REINFORC BARS
(P ₄₈)	48"	18"	(6) #5 BAF EACH WA
P ₅₁	48"	18"	(6) #5 BAF EACH WA

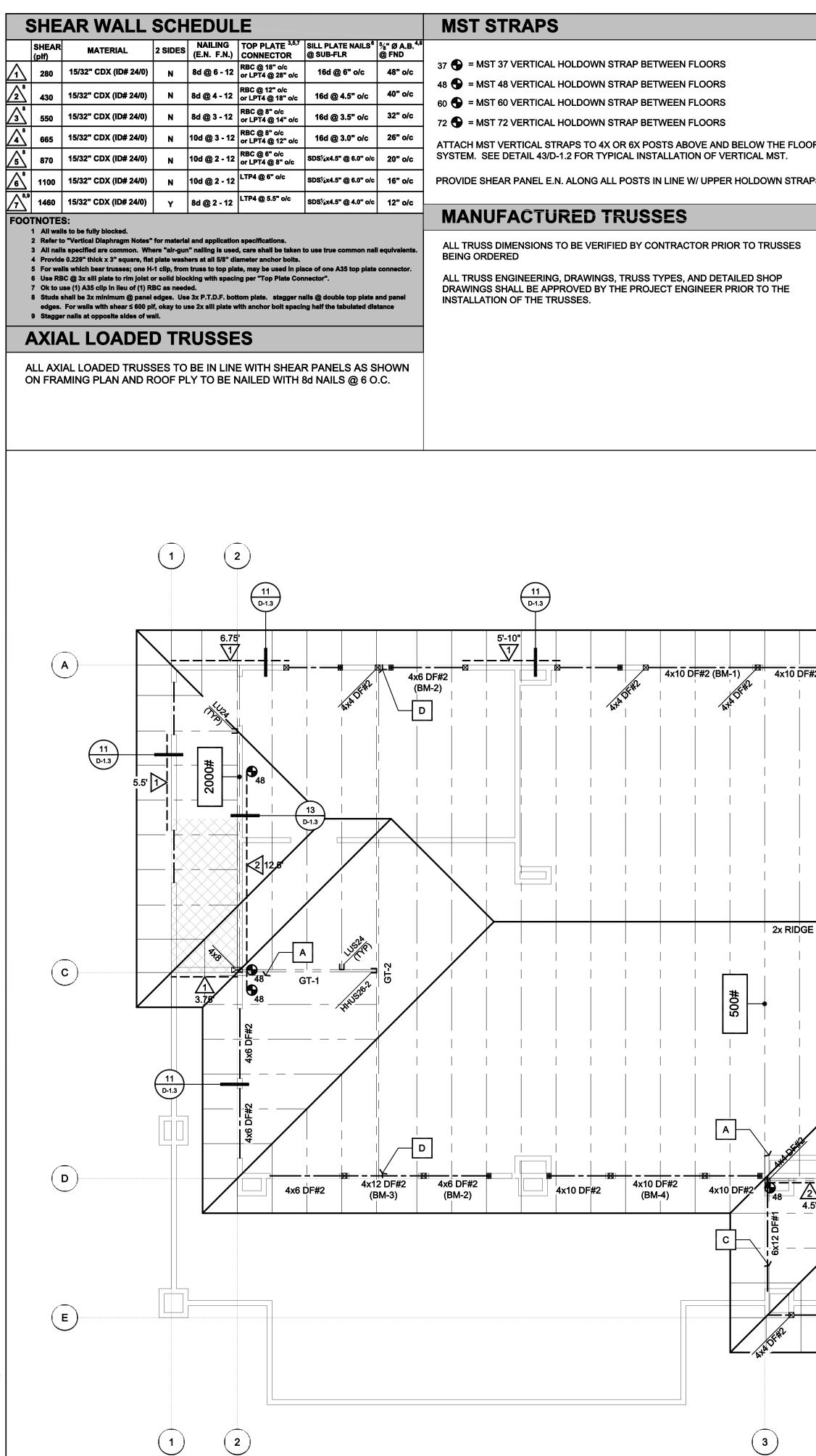


		SHE		SCH	EDUL	E			ΝΛΕ	Decigne
)/C. EACH WAY		SHEAR		2 SIDES	NAILING	TOP PLATE ^{3,5,7} CONNECTOR	SILL PLATE NAILS ⁶ @ SUB-FLR	⁵ ⁄8" Ø A.B. ^{4,8} @ FND		E. Designs
QUEEN. VISQUEEN	$\overline{\Lambda}$	(plf) 280	15/32" CDX (ID# 24/0)	N	8d @ 6 - 12	RBC @ 18" o/c or LPT4 @ 28" o/c	16d @ 6" o/c	48" o/c		Structural Engineering rafting & Design
FORCED PER TABLE	28	430	15/32" CDX (ID# 24/0)	N	8d @ 4 - 12	RBC @ 12" o/c or LPT4 @ 18" o/c	16d @ 4.5" o/c	40" o/c	610 10th Stree	
AME AS FINISHED M. USE #3 AND BENT 3'-0"	3^{8}	550	15/32" CDX (ID# 24/0)	N	8d @ 3 - 12	RBC @ 8" o/c or LPT4 @ 14" o/c	16d @ 3.5" o/c	32" o/c	Paso Robles, www.medesi	
		665	15/32" CDX (ID# 24/0)	N	10d @ 3 - 12		16d @ 3.0" o/c	26" o/c		Δ
S SHALL BE TO LOWEST GRADE.	5°	870	15/32" CDX (ID# 24/0)	N	10d @ 2 - 12	RBC @ 6" o/c or LPT4 @ 8" o/c	SDS ¹ / ₄ x4.5" @ 6.0" o/c	20" o/c	li li	SUP TOTOL
TURE CONTENT REQUIRED	<u>∧</u> 8,9	1100	15/32" CDX (ID# 24/0)		10d @ 2 - 12	LTP4 @ 6" o/c LTP4 @ 5.5" o/c	SDS ¹ / ₄ x4.5" @ 6.0" o/c	16" o/c	27.846	d 70724
NS & REINFORCEMENT VIDTH DEPTH BARS	7 ^{8,9} FOO ⁻	1460 TNOTE	15/32" CDX (ID# 24/0)	Y	8d @ 2 - 12		SDS¼x4.5" @ 4.0" o/c	12" o/c	R R	C 70724 ∰ Exp 06-30-11
12" 18" (2) #4 15" 18" (2) #4 18" 18" (2) #4	:	2 Refer	Ils to be fully blocked. to "Vertical Diaphragm Notes" Ils specified are common. Wh			-		egyhelente		TIF OF CALIFORNIT
		4 Provid 5 For wa	le 0.229" thick x 3" square, flat alls which bear trusses; one H	t plate wash -1 clip, from	ers at all 5/8" di truss to top pla	ameter anchor bolts. ate, may be used in pla	ice of one A35 top plate	-		
	1 .	7 Ok to	BC @ 3x sill plate to rim joist of use (1) A35 clip in lieu of (1) R shall be 3x minimum @ panel	BC as need	ed.			nd panel		
TS TS		-	a. For walls with shear ≤ 600 pl er nails at opposite sides of wa	· · · · · · · · · · · · · · · · · · ·	se 2x sill plate v	with anchor bolt spacing	ng half the tabulated dis	tance		
R FOOTING AND	i	LE\	/EL SHEAF	R BR		NOTE				
ND SPLICES K FLAT PLATE			ROVIDE ISB ILEVEL SH B CONC. TEMPLATE 1							
		<u>s∎∖</u> &:	SBA DETAIL SHEETS	FOR INS	STALLATION	NINSTRUCTION	S. USE A449			
		PL	ANS). NUTS SHALL E	BE HEAV	Y HEX AST	M A563 GR. DH.	WALL CAN BE			
			RDERED SEPARATEL ULTI-STORY KITS (MS					LLS,		
			0R iSB12x - FW=22, C1=9 0R iSB18x - FW=28, C1=1							
)R iSB24x - FW=35, C1=1							
										4
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									Ř	MRS. AGRAZ TRACT 2542 O GRANDE, C
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									Designs and s this project on	shall be used solely for the purpose of n this site. Any use other than the which it is intended for without the
										nt of M.E. Designs and Michael Shick
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									FILE NAME	S-1.0 FOUNDATION PLAN (GARAGE).DWG
									DRAWN BY DATE	M.SHICK 4/14/2011 11:04 AM
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										NDATION
									PLAN	
									(GAR	RAGE)
									SHEET NUMB	BER:
										S-1.0







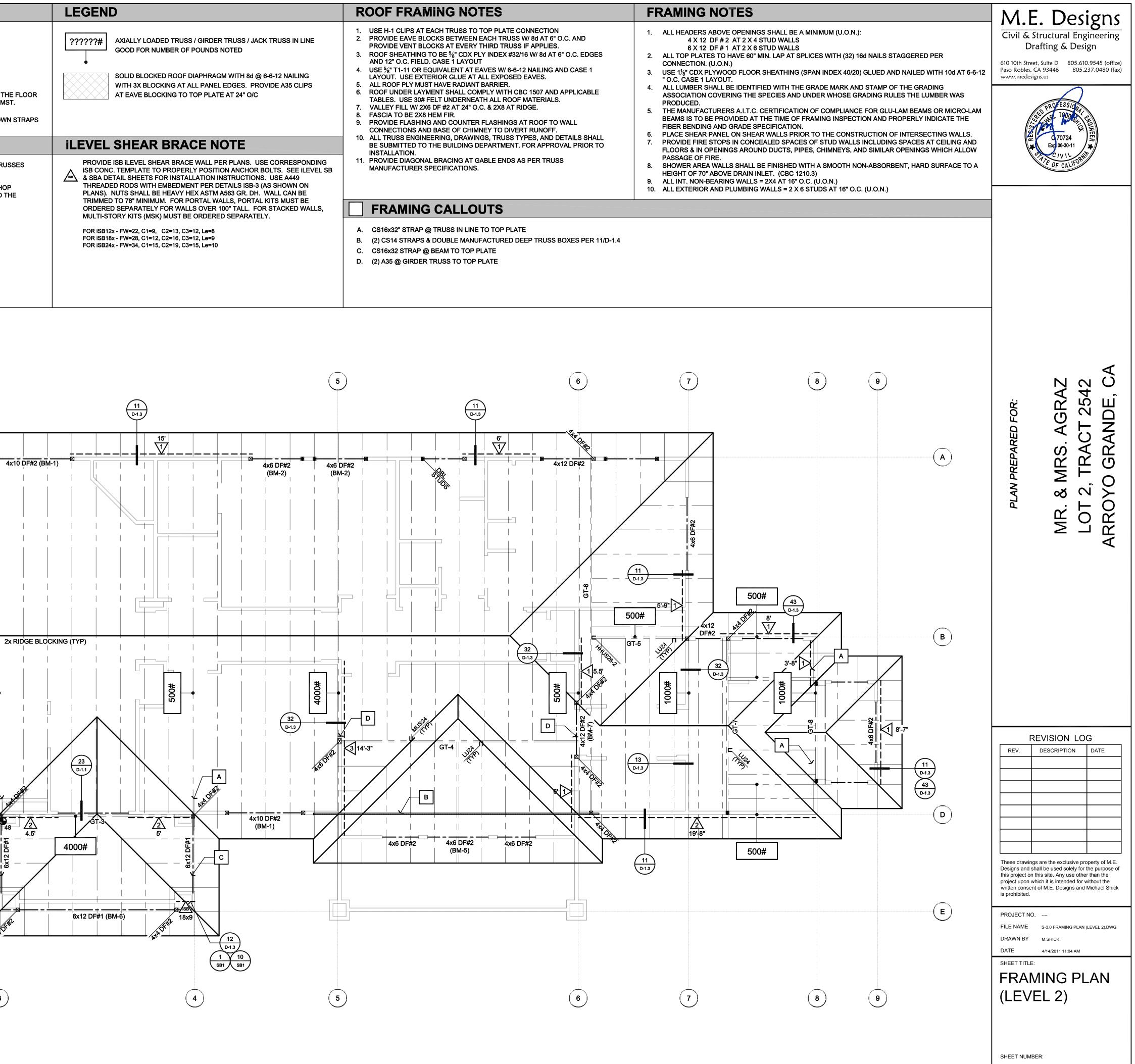


4x10 DF#2 (BM-1)

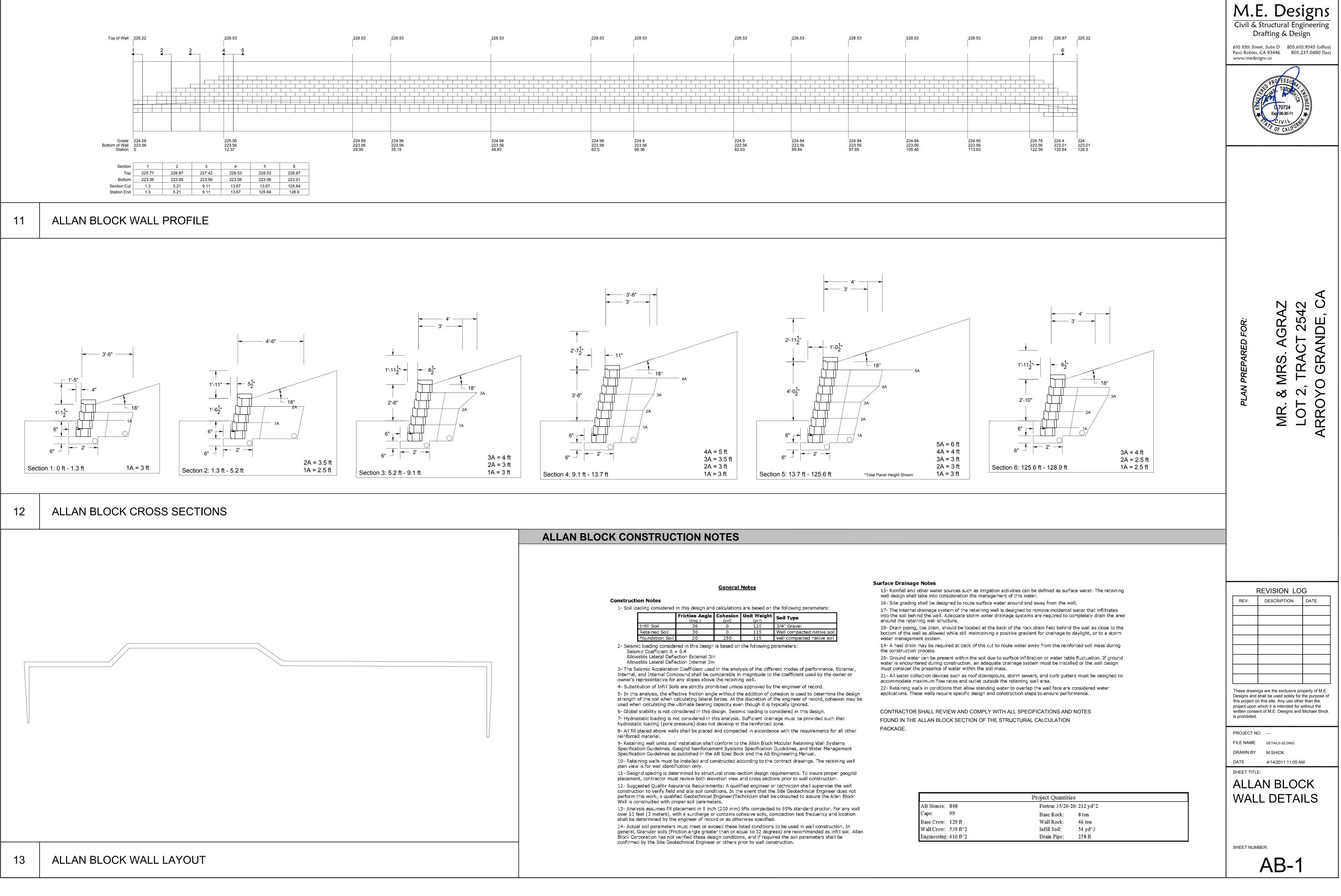
4x10 DF#2

3

	LEGEND	ROOF FRAMING NOTES
POR	?????# AXIALLY LOADED TRUSS / GIRDER TRUSS / JACK TRUSS IN LINE GOOD FOR NUMBER OF POUNDS NOTED SOLID BLOCKED ROOF DIAPHRAGM WITH 8d @ 6-6-12 NAILING WITH 3X BLOCKING AT ALL PANEL EDGES. PROVIDE A35 CLIPS AT EAVE BLOCKING TO TOP PLATE AT 24" O/C	 USE H-1 CLIPS AT EACH TRUSS TO TOP PLATE CONNECTION PROVIDE EAVE BLOCKS BETWEEN EACH TRUSS W/ 8d AT 6" O.C. AND PROVIDE VENT BLOCKS AT EVERY THIRD TRUSS IF APPLIES. ROOF SHEATHING TO BE %" CDX PLY INDEX #32/16 W/ 8d AT 6" O.C. EDGES AND 12" O.C. FIELD. CASE 1 LAYOUT USE %" T1-11 OR EQUIVALENT AT EAVES W/ 6-6-12 NAILING AND CASE 1 LAYOUT. USE EXTERIOR GLUE AT ALL EXPOSED EAVES. ALL ROOF PLY MUST HAVE RADIANT BARRIER. ROOF UNDER LAYMENT SHALL COMPLY WITH CBC 1507 AND APPLICABLE TABLES. USE 30# FELT UNDERNEATH ALL ROOF MATERIALS. VALLEY FILL W/ 2X6 DF #2 AT 24" O.C. & 2X8 AT RIDGE. FASCIA TO BE 2X8 HEM FIR. PROVIDE FLASHING AND COUNTER FLASHINGS AT ROOF TO WALL CONNECTIONS AND BASE OF CHIMNEY TO DIVERT RUNOFF.
	ILEVEL SHEAR BRACE NOTE	10. ALL TRUSS ENGINEERING, DRAWINGS, TRUSS TYPES, AND DETAILS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT. FOR APPROVAL PRIOR TO INSTALLATION.
Lise is Lise 8 T P	THREADED RODS WITH EMBEDMENT PER DETAILS ISB-3 (AS SHOWN ON PLANS). NUTS SHALL BE HEAVY HEX ASTM A563 GR. DH. WALL CAN BE	11. PROVIDE DIAGONAL BRACING AT GABLE ENDS AS PER TRUSS MANUFACTURER SPECIFICATIONS.
	TRIMMED TO 78" MINIMUM. FOR PORTAL WALLS, PORTAL KITS MUST BE ORDERED SEPARATELY FOR WALLS OVER 100" TALL. FOR STACKED WALLS, MULTI-STORY KITS (MSK) MUST BE ORDERED SEPARATELY.	FRAMING CALLOUTS
	FOR iSB12x - FW=22, C1=9, C2=13, C3=12, Le=8 FOR iSB18x - FW=28, C1=12, C2=16, C3=12, Le=9 FOR iSB24x - FW=34, C1=15, C2=19, C3=15, Le=10	 A. CS16x32" STRAP @ TRUSS IN LINE TO TOP PLATE B. (2) CS14 STRAPS & DOUBLE MANUFACTURED DEEP TRUSS BOXES PER 11/D-1.4 C. CS16x32 STRAP @ BEAM TO TOP PLATE D. (2) A35 @ GIRDER TRUSS TO TOP PLATE



S-3.0



2- Seismi Seism Allow Allow 3- The Se Internal, a	Infill Soil Retained Soil Foundation Soil c loading conside nic Coefficient A able Lateral Defi able Lateral Defi	Friction Angle (Deg.) 36 30 20 ered in this design = 0.4	Cohesion (psf) 0 0 250	s are based on t	the following parameters: Soil Type 3/4" Gravel	Surface Drainag 15- Rainfall and wall design shall 16- Site grading 17- The internal into the soil beh around the retai		
1- Soil loa 2- Seismi Seism Allow 3- The Se Internal, a	Infill Soil Retained Soil Foundation Soil c loading conside nic Coefficient A able Lateral Defi able Lateral Defi	Friction Angle (Deg.) 36 30 20 ered in this design = 0.4	Cohesion (psf) 0 0 250	Unit Weight (pcf) 120	Soil Type	16- Site grading 17- The internal into the soil beh		
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2- Seismi Seism Allow Allow 3- The Se Internal, a	c loading conside nic Coefficient A able Lateral Defle able Lateral Defle	ered in this design = 0.4	55 (PK)	115	Well compacted native soil well compacted native soil	bottom of the w		
Seism Allow Allow 3- The Se Internal, a	nic Coefficient A able Lateral Defi able Lateral Defi	= 0.4	- the file and the set of the set			water managem 19- A heel drain		
Allow 3- The Se Internal, a	able Lateral Defi	2- Seismic loading considered in this design is based on the following parameters: Seismic Coefficient A = 0.4 Allowable Lateral Deflection External 3in						
Internal, a	ismic Acceleratio	Allowable Lateral Deflection Internal 3in						
	3- The Seismic Acceleration Coefficient used in the analysis of the different modes of performance, External,							
orniar o ri	Internal, and Internal Compound shall be comparable in magnitude to the coefficient used by the owner or owner's representative for any slopes above the retaining wall.							
4- Substitution of Infill Soils are strictly prohibited unless approved by the engineer of record.						accommodate r 22- Retaining w		
5- In this analysis, the effective friction angle without the addition of cohesion is used to determine the design strength of the soil when calculating lateral forces. At the discretion of the engineer of record, cohesion may be used when calculating the ultimate bearing capacity even though it is typically ignored.								
		and a second				CONTRACTOR		
						FOUND IN THE		
hydrostatic loading is not considered in this analysis. Sufficient dramage must be provided such that hydrostatic loading (pore pressure) does not develop in the reinforced zone.								
 8- All fill placed above walls shall be placed and compacted in accordance with the requirements for all other reinforced material. 9- Retaining wall units and installation shall conform to the Allan Block Modular Retaining Wall Systems Specification Guidelines, and Water Management Specification Guidelines, and Water Management Specification Guidelines as published in the AB Spec Book and the AB Engineering Manual. 								
							10- Retaining walls must be installed and constructed according to the contract drawings. The retaining wall plan view is for wall identification only.	
11- Geogrid spacing is determined by structural cross-section design requirements. To insure proper geogrid placement, contractor must review both elevation view and cross sections prior to wall construction.								
12- Suggested Quality Assurance Requirements: A qualified engineer or technician shall supervise the wall construction to verify field and site soil conditions. In the event that the Site Geotechnical Engineer does not perform this work, a qualified Geotechnical Engineer/Technician shall be consulted to assure the Allan Block Wall is constructed with proper soil parameters.								
over 10 fe	eet (3 meters), w	ith a surcharge o	r contains co	phesive soils, co	mpaction test frequency and location			
14- Actual soil parameters must meet or exceed these listed conditions to be used in wall construction. In general, Granular soils (Friction angle greater than or equal to 32 degrees) are recommended as infill soil. Allan Block Corporation has not verified these design conditions, and if required the soil parameters shall be confirmed by the Site Geotechnical Engineer or others prior to wall construction.								
	6- Global 7- Hydros hydrostat 8- All fill p reinforced 9- Retaini Specificat Specificat 10- Retain plan view 11- Geog placemen 12- Sugge construct perform t Wall is co 13- Analy over 10 fe shall be d 14- Actua general, C Block Cor	 6- Global stability is not constructed in the stability of the st	 6- Global stability is not considered in this d 7- Hydrostatic loading is not considered in thydrostatic loading (pore pressure) does no 8- All fill placed above walls shall be placed reinforced material. 9- Retaining wall units and installation shall Specification Guidelines, Geogrid Reinforcen Specification Guidelines as published in the 10- Retaining walls must be installed and corplan view is for wall identification only. 11- Geogrid spacing is determined by struct placement, contractor must review both ele 12- Suggested Quality Assurance Requirement construction to verify field and site soil cond perform this work, a qualified Geotechnical Wall is constructed with proper soil parameters 13- Analysis assumes fill placement in 8 inclover 10 feet (3 meters), with a surcharge or shall be determined by the engineer of reco 14- Actual soil parameters must meet or exigeneral, Granular soils (Friction angle greater Block Corporation has not verified these desired and set of the set o	 6- Global stability is not considered in this design. Seisn 7- Hydrostatic loading is not considered in this analysis. hydrostatic loading (pore pressure) does not develop in 8- All fill placed above walls shall be placed and compact reinforced material. 9- Retaining wall units and installation shall conform to Specification Guidelines, Geogrid Reinforcement System Specification Guidelines as published in the AB Spec Boot 10- Retaining walls must be installed and constructed ad plan view is for wall identification only. 11- Geogrid spacing is determined by structural cross-si placement, contractor must review both elevation view. 12- Suggested Quality Assurance Requirements: A quali construction to verify field and site soil conditions. In th perform this work, a qualified Geotechnical Engineer/Te Wall is constructed with proper soil parameters. 13- Analysis assumes fill placement in 8 inch (200 mm) over 10 feet (3 meters), with a surcharge or contains co shall be determined by the engineer of record or as other 14- Actual soil parameters must meet or exceed these I general, Granular soils (Friction angle greater than or exceed Block Corporation has not verified these design condition 	 6- Global stability is not considered in this design. Seismic loading is considered in this analysis. Sufficient drain hydrostatic loading (pore pressure) does not develop in the reinforced at 8- All fill placed above walls shall be placed and compacted in accordan reinforced material. 9- Retaining wall units and installation shall conform to the Allan Block Specification Guidelines, Geogrid Reinforcement Systems Specification Specification Guidelines as published in the AB Spec Book and the AB E 10- Retaining walls must be installed and constructed according to the plan view is for wall identification only. 11- Geogrid spacing is determined by structural cross-section design replacement, contractor must review both elevation view and cross section 22- Suggested Quality Assurance Requirements: A qualified engineer or construction to verify field and site soil conditions. In the event that the perform this work, a qualified Geotechnical Engineer/Technician shall by Wall is constructed with proper soil parameters. 13- Analysis assumes fill placement in 8 inch (200 mm) lifts compacted over 10 feet (3 meters), with a surcharge or contains cohesive soils, conshall be determined by the engineer of record or as otherwise specified 14- Actual soil parameters must meet or exceed these listed conditions general, Granular soils (Friction angle greater than or equal to 32 degree Block Corporation has not verified these design conditions, and if require the conditions and if require these design conditions, and if require the conditions and the compacted in the section of the engineer of the engineer of the engineer of the section of the section of the engineer of the engineer of the section of the section of the section of the section of the engineer of the engineer of the section of the section of the section of the engineer of the section of the section	 6- Global stability is not considered in this design. Seismic loading is considered in this design. 7- Hydrostatic loading is not considered in this analysis. Sufficient drainage must be provided such that hydrostatic loading (pore pressure) does not develop in the reinforced zone. 8- All fill placed above walls shall be placed and compacted in accordance with the requirements for all other reinforced material. 9- Retaining wall units and installation shall conform to the Allan Block Modular Retaining Wall Systems Specification Guidelines, Geogrid Reinforcement Systems Specification Guidelines, and Water Management Specification Guidelines as published in the AB Spec Book and the AB Engineering Manual. 10- Retaining walls must be installed and constructed according to the contract drawings. The retaining wall plan view is for wall identification only. 11- Geogrid spacing is determined by structural cross-section design requirements. 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